How Low is too Low? Assessing the Risk of Low Air Voids using Accelerated Pavement Testing

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Low Air Voids

- During construction
 - Excess binder
 - Excess fines



- Too low
 - Plastic flow
 - Rutting and shoving under traffic
 - Flushing and bleeding
 - Increased maintenance
 - Shorter pavement life

What to Do?

- Remove and replace?
 Contractor risk mix might still perform
- Leave in place with reduced pay?
 - How much reduction?
 - DOT risk mix might fail

- How Low is Too Low?
- Design at 4% or 3-5%
- Foster in situ air voids ≤ 2.5% shoved
 - Instability at 3% for 4.75 mm DGA
- NCAT rutting mixes had air voids $\leq 3\%$
- Harvey and Tsai recommend design AV = 2% (perpetual pavement base)
- WesTrack minimal rutting in section with 1.6% air voids in situ

Factors Affecting Severity

Type of roadway, traffic level, climate

Depth within pavement structure

Strength/stiffness of mix

How do you know if it is safe to leave in place?

Indiana History

- Implemented Superpave in 1992-93
- Began volumetric acceptance of HMA in 2001
- Volumetric acceptance on all HMA in 2003
- Pay factors depend on binder content, VMA, air voids and density
- Plate sampling and density cores

Substandard Results

- If first sample "fails," backup sample is tested
- If backup sample also fails, suspect sublot is referred to Failed Materials Committee for disposition
 - Leave in place at reduced pay
 - Remove and replace

Concern

- Some sublots exhibited air voids <2%</p>
- Removal and replacement was indicated
- Costly for contractors (\$30/Mg × 1000 Mg)
- Testing variability issues and extenuating circumstances
- Wanted more objective way to determine action

Initiated Research

- Two Pronged Approach
 - NCAT Test Track 2006
 - INDOT/Purdue Accelerated Pavement Testing (APT) Facility
- Assess agency and contractor risk
- Recommend decision strategy for managing risk when accepting or rejecting low air void mixes

NCAT Phase III

- Sections S7 (A&B) and S8 (A&B)
 - 50 mm (2 in) surface removed and replaced with low void mix





Low QC Voids Experiment



APT Experiment



Air Voids in APT

Lane	Top 50 mm	Lower 50 mm	Cause
1	~4%	~2%	High binder
2	~4%	~2%	High fines
3 & 4	~2%	~4%	High binder

Each lane is 1.5 m (5 ft) wide and 6 m (20 ft) long.

355 to 430 mm (14 to 17 in.) pavement on 405 mm (16 in.) cement stabilized soil.





Rut Depths

5



Rutting [mm]

Rut Depths



Modeling

Rutting 'driving forces'

- Shear shape change
- Volumetric density change
- Subsystem approach
- A simple VP model

 $\dot{\varepsilon}_{ij}^{vp} = \frac{s_{ij}}{\eta_S} + \frac{p\delta_{ij}}{\eta_V}$



Resistance to volumetric deformation

 $s_{ij} = \sigma_{ij} - \frac{\sigma_{kk}}{2} \delta_{ij}$

 $p = \frac{O_{kk}}{c}$

Modeling

- Four layer system
- Assumed Poissson's ratio
- Backcalculated moduli from FWD
- Simulated moving wheel load
- Computed profiles compared to measured
- Refined model and simulation repeated

Modeled Rut Depth





Decision Support Tool

AVC [%]	Traffic intensity (20 year)		
	Low	High	1 - Accent
3.0	1	1	2 = Reduce Pay
2.9	1	2	3 = Reject
2.8	2	2	
2.7	2	2	
2.6	2	3	
2.5	3	3	

Monetary Reduction

- How to determine appropriate pay reduction?
- Assess impact on life cycle
 - QRSS (NCHRP 9-22) based on MEPDG
 - As-designed vs. As-built
 - How much was life cycle reduced?
 - Rough rule of thumb \$10,000/lane mile/yr
- Analysis in progress

Conclusions

- Currently air void levels below 2-3% appear problematic regardless of position.
- Cause of the low voids does not matter.
- Risk to agency and contractor.
- Preliminary decision support tool being refined to consider impact on service life.
- Drilled holes provide some insight need refinement.

For more information:

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